ROY F. WESTON, INC.

ENVIRONMENTAL SCIENTISTS AND ENGINEERS

ORIGINAL (Red)

9 January 1973

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Mr. Lester F. Meyer Delaware Division of Environmental Control Water Pollution Section Tatnali Building Dover, Delaware 19901

Dear Duffy:

Enclosed is a copy of our latest report on the ground water pollution situation at the Liangollen Landfill in New Costle County, proposed work outlined at the end of the report will begin the of January 15th with drilling and testing of potential leachate overy wells between the landfill and the Artesian Water Company well field. Please do not hesitate to contact me with your comments.

Sincerely yours,

Michael A. Apgar

P.S. I have also attached a tabulation of our water analyses results on the samples you collected at Llangollen with Roger Gresh and Rark Horner on December 19th. Wells DC 14-13 and DC 24-6 are observation wells belonging to Imace Chemical Corporation. Their locations are shown on Figure 3 of the enclosed report. Neither well had been previously sampled, and I understand that you had to rush your samples back to the lab and were not present when the two wells were pumped late that afternoon.

Well 14-13 is uncontaminated, because it is probably not in the leachate flowpath and also because it is screened in a relatively clayey unit along which leachate would probably not have invaded anyway.

Well 26-6 is slightly contaminated—either by the Llangollen Landfill or the Deloware Sand and Gravel Co. fill. If from Llangollen, contaminants have moved at least 2000 ft. through the subsurface to date.

I would appreciate receiving copies of your analytical results on the other water samples collected that day. I will need permits for the proposed wells to be drilled next week. I will be interpreted with you in a few days when the exect starting date for drilling to 04 firmed up.

Roy F. Weston Results of Analyses on Water Samples Collected Jointly With DEC on December 19, 1972, From Wells in the Vicinity of the Llangollen Landfill:

<u>We11</u>	Temperature	Specific Conductance (umhos/cm)	Cl (ppm)	Conments
Robelens	61	69	11	Clear, odorless
1A	57	90	11	Clear, odorless
3.4	61	1100	128	Slightly turbid, (milky) and foul odor
Dc 14-13	56	54	6	Slightly turbid odorless
Dc 24-6	57	170	30	Gray-brown tinge with slight turbidity, slight organic (oily) odor

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Proceedings of the Late of the Control Control

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Duclosed is a copy of a reject on the Unagolica Landfill Siruction tolch cover, such consultangle the cod of September, 1972. This report include all becomes diagrams information and water quality confyces cottons never.

Inour ation from the Conter/Hovenhor test drilling project which to the discount in your collection bedeemer th, 1972, is being typed, to encode reject included the extent of contribution in the took of Saulier, estimates on the rate of contribution between, and the classes, which is contributions reach the caper passing wills. The research is such a contribution of corrected procedures is lading in the case of the contribution in lading in the case of the contribution is such as the case of the covery wills. The every very contribution work should be in a consequent to

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GROUND WATER CONTAMINATION ASSOCIATED WITH THE ELANGOLLEN LAMBFILL, NEW CASTLE COUNTY, DELAWARE

EXTENT OF CONTAMINATION AND PROPOSED CORRECTIVE PROCEDURES

JANUARY 1973

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The Llangollen Landfill was constructed from 1960 to 1968 in a depleted sand and gravel quarry. The landfill is approximately 4400 feet long, 250 to 900 feet wide and 30 to 50 feet thick. The location of the landfill is shown in Figure 1.

Hydrogeologic Setting

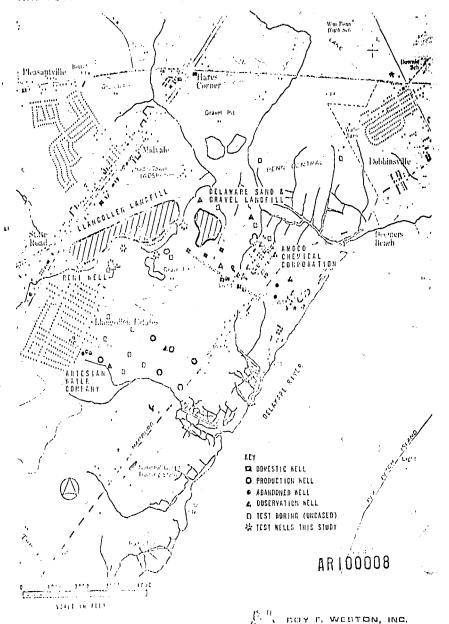
The sand excavated from the quarry was Pleistocene age. In the Llangollen area these sands, known as the Columbia Formation, form a nearly continuous surficial cover up to 60 feet in thickness. The base of the formation ranges from about 10 feet above to 20 feet below mean sea level in the vicinity of the landfill.

The underlying Potomac Formation consists of stream-deposited unconsolidated sands, silts and clays of Lower Cretaceous Age. The sand units are channel-shaped, with extensive interbedded lenses of clay and silt which accumulated on ancient floodplains. The Potomac Formation thickens and dips towards the southeast at approximately 40 to 140 feet per mile (uppermost and lowermost beds respectively) in the study area.

Pydrologically the generally coarse Columbia deposits serve as an infiltration and recharge gallery for the Potomac sands: Ground water in the Potomac sands becomes confined (artesian) beneath relatively impermeable beds of clay and silt as it travels seaward down dip in the formation. The approximate thickness of the Potomac confining units immediately beneath the Columbia sands are shown in figure 2. Immediately beneath the landfill these clay and silt deposits are shallow, thin order the area off the southeast corner of the fill-passent.

Landfill construction in the open pit proceeded from east to west. Refuse covering operations were conducted by the quarry operator, using the quarrying equipment which was already on the site. Nearly all intermittent cover material was obtained within the pit from waste piles and siltation basins. As time progressed, cover paterial and landfill space became critically depleted 100007 on which encouraged decour expandion on the western end of the pit. This excavation removed some ready in a few rior is probably allocof the confining clay on top of

TO LITION OF WILLIAMSTON SOUTH U.S.G.S. 7% PINNITE TOPOCHAPHIC MAP SUDVICION THE LOCATIONS OF THE LEANGOLLEN LANDFILL ALL MELLS AND BOLINGS IN ITS VICINITY, SEPTEMBER, 1972



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the Potomac sends. Such conditions permitted direct access to the Potomac sands by leachate from the landfill.

Ground Water Movement

Under natural conditions, water entered the Potomac sands where they subcropped beneath the permeable Pleistocene sediments. This water moved slowly downdip in the formation to be discharged to the Delaware Estuary or Atlantic Ocean by vertical leakage. However, the rate of ground water movement has been greatly accelerated locally by heavy pumpage by the Amoco Chemical Corporation (about 2.5 mgd) and the Artesian Water Company (about 4.5 mgd). The locations of both these well fields are shown on Figure 3.

In August and September, 1972, 10 test wells were drilled to different depths at 4 locations in the vicinity of the landfill. During October and November, the perfect test borings were made and completed with piezometers for water placed measurement. A generalized piezometric map of the Potomac sands utilizing that a from measurements made in these wells and a few industrial observation wells in September and December, 1972, and the major production wells in September is shown in Figure 3. The theoretical direction of ground water movement (shown on the figure) is perpendicular to the piezometric contours. However, in actuality, ground water will tend to move along the sand and gravel-filled channels—avenues along which the permeability is greatest. Unfortunately, both the piezometric gradient and the permeable channels trend towards the high capacity pumping wells.

The average rate of movement of a slug of water through the aquifer, can be approximated by the expression:

The maximum coefficient of permeability along the Potomec channel deposits is probably on the order of 1000 \gcd/ft^2 (the permeability of a clean, fine gravel).

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FIGURE 3 - MAP OF THE LEAGUAGES IN THE UPPER POTOMAC AQUIFER. ORIGINAL (Rod) WATER FLOW DIRECTIONS IN THE UPPER POTOMAC AQUIFER. ALSO SHOWN ARE WATER QUALITY LITERPRETATIONS FOR WELLS AND TEST BORINGS AND THE LOCATIONS OF PROPOSED TEST BORINGS AND COLLECTION MELLS. theore and direction of gram' sinter By an particular and Lear. "amountable well or borney is mitamineted well or contact 200 Gest Z.

The piezometric gradient increases towards the pumping wells such that the velocity of a slug of water will increase as the wells are approached.

The approximate time required for water to move along its flow path can be calculated from the equation:

(2) $t = \overset{d}{V}$, where t = travel time d = length of flowpath <math>v = average apparent velocity

Approximate flow velocities and travel times of ground water moving from the landfill to major pumping wells are listed in Table 1. These values have been calculated from Equations 1 and 2 (above) assuming a constant coefficient of permeability of 1000 gpd/ft².

There results represent maximum ground water velocities and minimum travel times. Actual rates of flow will be lower where the permeability of the inconsolidated media is less than 1000 gpd/ft². The coefficients of permeability of most of the sand encountered during test drilling is estimated at closer to 500 gpd/ft², although some gravels had K's of at least 1000 gpd/ft². Contaminants from the landfill which enter the aquifer will move most rapidly along the coarsest deposits, while simultaneously spreading throughout the less permeable sections of the aquifer.

The calculations indicate that contaminants leached from the landfill would reach Amoco Well PW-2 in six and a half years. This well has been in production since 1962 so that the gradient between the well and the landfill has probably existed similar to the present situation for more than the calculated travel time. Thus, if the assumptions behind the calculations are correct, contaminants should already have reached the well. However, contaminant travel may have been retarded by sands of relatively low permeability and/or the absence of a direct channel connection between the sands underlying the landfill and those from which water is being withdrawn. In addition, fresh recharge could gain access to the Potomac sands midway along the flowpath in the area where the confining beds

TABLE 1 ESTIMATED GROUND WATER VELOCITIES AND TRAVEL TIMES BETWEEN THE LLANGOLLEN LANDFILL AND MAJOR PRODUCTION WELLS IN THE VICINITY

Landfill to Amoco Chemical Corp. Well PW-2 (Flowpath Length = 3360 ft.):

Flowpath Section	1 Average	V Average (ft/d)	Approximate t (Days)
Landfill- 9601 9601-24001 24001-28801	.013 .014 .021 .042	1.73 1.86 2.80 5.60	555 774 171 43
-31201 -33601	.083	11.0	22 1565d=4.29 yrs.

Landfill to Artesian Well Co. Well E-2 (Flowpath Length = 3350 ft.):

Flowpath Section	<u> Average</u>	V Average (ft/d)	Approximate t (Days)
Landfill- 720' 720'-1360'	.007	0.93	774
13601-19201	.008 .009	1.07	598 467
1920'-2720' 2720'-3000'	.013 .036	1.73 4.80	462 58
5" -3150" 3150"-3350"	.057 .100	8.93 13.3	17 15
	,		2391d=6.55 yrs.

Landfill to Artesian Well Co. Well #7 (Flowpath Length = 3000 ft.):

Flowpath Section	<u> 1 Average</u>	V Average (ft/d)	Approximate t (Days)
Landf111-10501	,008	1.07	981
10501-21001	,010	1.33	789
21001~28001	,014	1.87	374
28001-30001	.033	4.40	45
1			2189d=6.00 yrs.

are absent, thus improving the quality of water reaching the well. At present, some of this area has been covered by the Delaware Sand & Gravel Company landfill (which can be expected to contribute contaminants to the Potomac aguifer).

The Artesian Water Company has operated large capacity wells at Llangollen Estates since 1953, gradually expanding the number of wells and amount of pumpage to present. Well 7 was constructed on the eastern edge of the old well field in 1969. It is the only well in the old well field still in regular use. The other old wells were replaced by the new well field (including Well E-2) to the east which went into service in 1971. Thus, the steep plezometric gradients towards these wells are a recent development, and projected contaminant travel times must be extended to account for this condition.

Extent of Ground Water Contamination

the trained on the basis of analysis of water samples collected from virtually all of the wells in the landfill vicinity in August and September, 1972 and water quality interpretations of geophysical logs run in the test borings during October and November, 1972. The results of water analyses are listed in Table 2. Landfill leachate had affected some well waters mainly as objectionable taste and odor with only slight increases in dissolved solids and dissolved oxidizable material, while in a few cases, dissolved solids and chemical oxygen demand were extremely high and odor was overpowering.

tocations where ground water contamination has been proven or is suspected are shown in Figure 3. The figure indicates that contaminants have, in certain places, already travelled past the test holes most distant from the landfill. It is estimated that contaminants may be about 2000 feet along the flowpath to Artesian Water Company Well E-2. If so, the contaminants can be expected to reach the well within 400-500 days. Contaminants may also have moved more than 2000 feet from the landfill past the Reni well, but reference to the plezometric map makes it appear that this material is also moving towards the new well field.

TABLE 2

CHEMICAL QUALITY OF GROUND . SER SAMPLES FROM THE POTOMAC FORMATION IN THE VICINITY OF THE LLANGOLLEN LANDFILL

Well.	Date Sampled	Organic Odor	Specific Conductance (4mhos/cm)	(maa)	(maa)	ΣFe (visual est.)
Aroup Film Plant #2	9-22-72	~ none	100	19	33.1	
Acoco Film Plant #3	9-22-72	~ none	33	10	15.9	some :
Aroso Film Plant (4)	9-22-72	onon.	120	4 5	13.3	∜ amos
Amodo Polynar Plant PW-1	9-22-72	- попе	170	9	13.4	→ none
£roco Polymer Plant PW-2	9-22-72	~ none	160	29	45.0	~ none
Asoco Polymer Plant PW-3	9-22-72	~ none	110	4.5	21.3	none
Artesian Water Co. #7	9-28-72	~ none	58	Q	7.3	none
Artesian Water Co. E-2	9-28-72	ouou ∼	57	18	8.9	~ none
Artesian Wator Co. 6-3	5-28-72	~ none	94	0	4.4	onon ~
Artesian Water Co. K-1	9-28-72	~ попе	841	18	4.4	~none (∵
	9-22-72	~ none	120	4 5	22.0	~ none
AF	9-22-72	\sim none	125	10	26.9	~ none
₹10	9-22-72	v. slight	100	7 2	24.1	ORIG (Re
00	9-22-72	v. slight	125	15	28.7	INAL ed)
5 apper associated	9-22-72	strong	140	19	29.5	heavy

		>		, , , , , , , , , , , , , , , , , , ,	·				
Σ Fe (visual est.)	SOME	heavy	÷r0a ~	tveed .	~ none	+	+	+	+
(1)	67.4	69.2	18.2	124.	19.4	15.9	254.	30.5	43.7
000	ō,	9	7 5	7 1000	ł	20	64	30	56
Specific Conductance	350	350	80	1300	100	250	1500	210	240
Organic	strong	strong	~ none	v. streng	- none	none?	v. slight?	~ none	v. slight
Date Sampled	9-22-72	9-22-72	9-22-72	9-22-72	8-8-72	8-30-72	9-1-72	9-7-72	9-28-72
<u> </u>	V. Dell Averseno	J. Beil Aversano	Sectodes	7.ch.	Test Well 1A	Test Woll 25 0	Tost Well 3A	Tost Well 40	Test Wall 40

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or may be due to infrequent well use

- masked by high turbidity

orange may be affected by residual development solution

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PROPOSED CORRECTIVE PROCEDURES

The present high rate of leachate generation and migration from the Llangollen Landfill must be curtailed immediately. The longer corrective procedures are delayed, the greater the quantity of leachate which must be recovered and treated and the greater the volume of aquifer contaminated—and the greater the expense. Thus, starting in January, 1973, we propose to conduct the following work:

- 1. Drill 4 wells into the landfill to monitor and control water levels in the refuse.
- Drill 3 test borings north of the landfill and install piezometers to monitor water levels in the Pleistocene sands and the piezometric heads in the Potomac aguifer.
- Drill 4 wells immediately south of the landfill to recover leachate from the Potomac Formation which is still close to and concentrated from the landfill.
- 4. Drill 4 wells at the expected furthest extent of leachate movement towards the Artesian Water Co. well fields for contaminant monitoring and/or recovery.

The locations of all the proposed wells and test borings are shown on Figure 3.

These wells will be pump tested to determine aquifer characteristics and water quality. Wells which can recover contaminants from the aquifer will be designated — for that purpose--plans for the disposal of recovered contaminants to be developed when the picture becomes clearer.

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*Ne contamination has been positively identified off the western end of the landfill. Hopefully, the confining beds have not been breached beneath this part of the landfill, and the aquifer will not become contaminated here. Perhaps the Artesian old well field (Well #7) will not be contaminated—at least as long as the new well field continues to withdraw larger quantities of water.

Distributing the present rate of pumpage over a wider area could reduce the hydraulic gradient—and rate of contaminant movement—towards the new well field. This could be accomplished by putting a well at the western end of the old well field (perhaps Well #2) back into production while cutting back on the pumping rates of Wells E-2 and K-1. Although this would allow more time for contaminant interception, such a procedure would result in contamination of a broader portion of the aquifer than is expected under present conditions.

Amoco Well PW-2 has higher than background concentrations of chloride and chemical oxygen demand. These parameters, while not present in sufficient concentrations to be troublesome may have originated at the landfill. Water quality in this well is expected to deteriorate, especially because (as previously mentioned) leachate from the Delaware Sand and Gravel Co. should enter the sandy part of the aquifer, which once served as an infiltration gallery for fresh recharge.